[1]. The diagram below shows the outlet from a water drainage system consisting of a main line (1) and a secondary line (2) draining through an outlet pipeline (3) located downstream from a pipeline expansion. The main and secondary lines are flowing full (i.e., under pressurized conditions). The diameters of the main line (1) and of the secondary line (2) are 2.0 ft and 1.0 ft, respectively. At the outlet pipeline (3) the flow is under open-channel conditions, and the water depth there is half the diameter of the pipeline. The flow velocities at sections (1) and (2) are 2.5 ft/s and 3.2 ft/s, respectively. (a) Determine the total discharge draining out of the main pipeline at section (3); (b) Determine the flow velocity at section (3);



Solution (a):

Q3:=Q1+Q2 Q3=10.3673 cfs

From the figure: $D3 = 2 \cdot 1.5$ ft

 $A3 \coloneqq \frac{1}{2} \cdot \left(\frac{\pi \cdot D3^2}{4}\right) \qquad A3 \equiv 3.5343 \text{ ft}^2$ Solution (b): $V3 \coloneqq \frac{Q3}{A3} \qquad V3 \equiv 2.9333 \frac{\text{ft}}{\text{s}}$

[2] A Venturi meter is placed on a pipe at an angle of 450, as shown in the figure below. (a) Using the manometer equation, determine the pressure difference $\Delta p = p2 - p1$, in psi. (b) If the diameters of the Venturi meter are D1 = 8 in and D2 = 4 in, determine the discharge through the Venturi meter. Neglect all energy losses.

water Я FLOW 8 in 8 sin(45°) v 6.5 in Φ-45° S = 13.56 (mercury) Energy terms in points (1) and (2) Point (1): Point (2):

Manometer equation:

 $p2 + 62.4 \cdot \left(\frac{8}{12}\right) \cdot \sin\left(\frac{\pi}{4}\right) + 62.4 \cdot y + 13.56 \cdot 62.4 \cdot \left(\frac{6.5}{12}\right) - 62.4 \cdot \left(\frac{6.5}{12}\right) - 62.4 \cdot y = p1$

The term "62.4*y" is cancelled, thus the equation reduces to:

$$p2 + 62 \cdot 4 \cdot \left(\frac{8}{12}\right) \cdot \sin\left(\frac{\pi}{4}\right) + 13 \cdot 56 \cdot 62 \cdot 4 \cdot \left(\frac{6 \cdot 5}{12}\right) - 62 \cdot 4 \cdot \left(\frac{6 \cdot 5}{12}\right) = p1$$

From which, we can calculate:

$$\Delta p = p2 - p1 = 62 \cdot 4 \cdot \left(\frac{6 \cdot 5}{12}\right) - \left(62 \cdot 4 \cdot \left(\frac{8}{12}\right) \cdot \sin\left(\frac{\pi}{4}\right) + 13 \cdot 56 \cdot 62 \cdot 4 \cdot \left(\frac{6 \cdot 5}{12}\right)\right)$$

$$\Delta p := \left(62 \cdot 4 \cdot \left(\frac{6 \cdot 5}{12}\right) - \left(62 \cdot 4 \cdot \left(\frac{8}{12}\right) \cdot \sin\left(\frac{\pi}{4}\right) + 13 \cdot 56 \cdot 62 \cdot 4 \cdot \left(\frac{6 \cdot 5}{12}\right)\right)\right)$$

$$\Delta p = -453 \cdot 9436 \text{ psf}$$
To convert to "psi" divide by 144: $\Delta p \text{ psi:=} \frac{\Delta p}{144}$
Solution (a): $\Delta p \text{ psi=-} 3 \cdot 1524 \text{ psi}$
Equation of continuity (1) - (2): $Q = \frac{\pi \cdot D1^2}{4} \cdot V1 = \frac{\pi \cdot D2^2}{4} \cdot V2$
i.e., $D1^2 \cdot V1 = D2^2 \cdot V2$, from which: $V2 = \left(\frac{D1}{D2}\right)^2 \cdot V1$
Also, $\frac{V2}{V1} = \left(\frac{D1}{D2}\right)^2$

z1:= 0
$$z_{2}:=z_{1}+\left(\frac{8}{12}\right)\cdot\sin\left(\frac{\pi}{4}\right)$$
 , i.e., z2=0.4714 ft , and $\Delta z=z_{2}-z_{1}=0.4712$ ft

p1=? p2=? We do know, that $\Delta p = p2 - p1 = -453.9436 \text{ psf}$

V1=? V2=? We do know, that
$$V2 = \left(\frac{D1}{D2}\right)^2 \cdot V1$$

so, that $\frac{V2^2}{2 \cdot g} - \frac{V1^2}{2 \cdot g} = \frac{V1^2}{2 \cdot g} \cdot \left[\left(\frac{V2}{V1}\right)^2 - 1\right] = \frac{V1^2}{2 \cdot g} \cdot \left[\left(\frac{D1}{D2}\right)^4 - 1\right]$

Bernoulli Equation (1)-(2):
$$z1 + \frac{p1}{\gamma} + \frac{V1^2}{2 \cdot g} = z2 + \frac{p2}{\gamma} + \frac{V2^2}{2 \cdot g}$$

$$\left(z2-z1\right)+\left(\frac{p2-p1}{\gamma}\right)+\left(\frac{v2^{2}}{2\cdot g}-\frac{v1^{2}}{2\cdot g}\right)=0$$

i.e.,
$$\Delta z + \frac{\Delta p}{\gamma} + \frac{V1^2}{2 \cdot g} \cdot \left(\left(\frac{D1}{D2} \right)^4 - 1 \right) = 0$$

with
$$\Delta z = z^2 - z^1$$
 i.e, $\Delta z = 0.4714$, $\Delta p = -453.9436 psf$

$$D1 := \frac{8}{12}$$
 ft, or, $D1 = 0.6667$ ft, also, $D2 := \frac{4}{12}$ ft, or, $D2 = 0.3333$ ft

$$\gamma = 62.4 \frac{1b}{ft^3}$$
 and, $g = 32.2 \frac{ft}{s^2}$, we solve for V1 by using:

solve
$$\left(\Delta z + \frac{\Delta p}{\gamma} + \frac{V1^2}{2 \cdot g} \cdot \left(\left(\frac{D1}{D2}\right)^4 - 1\right) = 0$$
, $V1 = \begin{pmatrix} -5.4045 \\ 5.4045 \end{pmatrix}$

Select: V1:= 5.4045
$$\frac{\text{ft}}{\text{s}}$$
, then Q:= V1 $\cdot \left(\frac{\pi \cdot \text{D1}^2}{4}\right)$, i.e.,
Solution (b): Q=1.8865 cfs

[3] The figure below shows a pump P lifting water from a pond through a 6-in-diameter suction pipeline and delivering it at a velocity of 2.5 fps through a 12-in-diameter discharge pipeline. The suction pipeline is provided by a trash screen, S1, with a minor loss coefficient KS1 = 0.6, and one elbow, E1, with a minor loss coefficient KE1 = 1.2. As shown in the figure, the delivery pipeline is fitted with two elbows, E2 and E3, with discharge coefficients KE2 = KE3 = 0.8. The pump-pipeline system is provided with two pressure gages: G1, located in the suction end of the pump, and G2, located after the second elbow in the discharge pipeline and 3.5 ft above the pump. Gage G2 shows a reading of 2.0 psi. Determine (a) the power that the pump delivers to the flow in horsepower; and (b) the pressure in gage G1 in psi.

3.5 ft

6.3 ft

1.2 ft

E1

Ø 6 in.

using the equation:

🔿 G2

E2

E3 _

Ø 12 in.



hm= Km
$$\cdot \frac{v^2}{2 \cdot g}$$

where Km is the corresponding minor loss coefficient and V is the mean velocity in the pipeline where the fitting (screen, or elbow) is located. The friction losses in a pipeline of length L and diameter D are calculated using the equation:

$$hf = f \cdot \frac{L}{D} \cdot \frac{V^2}{2 \cdot g}$$

where f is a friction factor, and V is the velocity in the pipeline. For the 6-in pipeline in the figure the friction factor is f6 = 0.021, while for the 12-in pipeline the friction factor is f12 = 0.012.

zC:= 6.3 ft VC:= V1 VC= 10 fps pC=?

Minor losses (A)-(C): hm=hS1+hE1 hm=2.795 ft

Friction losses (A) - (C): hf=hf1 hf=0.6848 ft

Energy equation (A) - (C):

$$zA + \frac{pA}{\gamma} + \frac{VA^2}{2 \cdot g} - hm - hf = zC + \frac{pC}{\gamma} + \frac{VC^2}{2 \cdot g}$$

$$pC \coloneqq \gamma \cdot \left(zA + \frac{pA}{\gamma} + \frac{VA^2}{2 \cdot g} - hm - hf - \left(zC + \frac{VC^2}{2 \cdot g} \right) \right)$$

pC psi= $\frac{pC}{144}$ pC psi=-4.9108psi

$$hf2 := f2 \cdot \frac{L2}{D2} \cdot \frac{V2^2}{2 \cdot g}$$
 i.e., $hf2 = 0.0093$ ft

Point (A): surface of pond, Point (B): location of gage G2 VA:= 0 pA:= 0 zA:= 0 VB:= V2 , i.e., VB= 2.5 $\frac{ft}{s}$ рВ:**=** 2 · 144 zB=6.3+3.5 , i.e., zB=9.8 ft pB=288 psf Total minor losses(A)-(B): hm=hS1+hE1+hE2+hE3 hm=2.9503 ft Total friction losses (A) - (B): hf=hf1+hf2 hf=0.6941 ft Energy equation (A) - (B):

$$zA + \frac{pA}{\gamma} + \frac{VA^{2}}{2 \cdot g} - hm - hf + hP = zB + \frac{pB}{\gamma} + \frac{VB^{2}}{2 \cdot g}$$

$$hP := solve \left(zA + \frac{pA}{\gamma} + \frac{VA^{2}}{2 \cdot g} - hm - hf + hP = zB + \frac{pB}{\gamma} + \frac{VB^{2}}{2 \cdot g} , hP \right)$$
Then, $hP = 18.1568$ ft
Power delivered by pump: $Q := V1 \cdot \left(\frac{\pi \cdot D1^{2}}{4} \right) \quad Q = 1.9635$ cfs

 $Pp := \frac{\gamma \cdot Q \cdot hP}{550} \qquad Pp = 4.0448 \qquad horsepower$

Problem [4]. (Take home). The figure below shows a 900 reducing elbow located in a vertical plane that delivers water to an outlet at section (2) open to the atmosphere. The diameters of sections (1) and (2) are 2.0 m and 1.0 m, respectively. A pressure gage at section (1) reads a value of 200.0 kPa. Determine (a) the water discharge through the elbow, Q; (b) the x-component of the force that the flowing water applies on the elbow; and, (c) the y-component of the force that the flowing water applies on the elbow. Assume negligible energy losses.

NOTE: The volume of water contained within sections (1) and (2) is not known, therefore, you need to provide a reasonable guesstimate for this volume from the information in the figure (e.g., using two cylinders). The volume of water between sections (1) and (2) is required to estimate the weight of the water for the momentum equation.

D1:= 2.0 m	D2:=1.0 m	$g \coloneqq 9.81 \frac{m}{s^2}$	$\gamma \coloneqq 9810 \frac{N}{m^3}$
Continuity: $\frac{\pi \cdot D1^2}{4} \cdot V1 = \frac{\pi \cdot D2^2}{4} \cdot V2 \implies V1 = \left(\frac{D2}{D1}\right)^2 \cdot V2$			
with $\left(\frac{D2}{D1}\right)^2$	=0.25 , the	n, V1=0.25	V2



Energy (1)-(2) with no losses, i.e., Bernoulli's equ:
Point (1):
$$zl=0$$
 $pl=20010^{-3}y_{a}$ $vl=2$
Formulli's equation: $zl+\frac{p1}{q}+\frac{vl}{2\cdot q}=z2+\frac{p2}{q}+\frac{v2^{2}}{2\cdot q}$
Bernoulli's equation: $zl+\frac{p1}{q}+\frac{vl}{2\cdot q}=z2+\frac{p2}{q}+\frac{v2^{2}}{2\cdot q}$
Following for V2:
 $v2=aclve\left[zl+\frac{p1}{q}+\frac{(0.25\cdot v2)^{2}}{2\cdot q}-\left[z2+\frac{p2}{q}+\frac{v2^{2}}{2\cdot q}\right], v2, 0, 100\right]$
 $v2=17.1313\frac{n}{a}$ $v1=0.25\cdot v2$ $v1=4.2028\frac{n}{a}$ Discharge: $0=v2\left[\frac{n\cdot D2^{2}}{4}\right]$ $0=13.4588\frac{n^{3}}{2}$ $p=1000\frac{k\pi}{m^{3}}$
Bothered volume of water $v0l=\frac{n\cdot 0l^{2}}{4}$ $5+\frac{n\cdot 02^{2}}{4}$ $-i$ $-i$ $vol=18.8496$ n^{3} Weight: $w=v.vol$ $w=1.8492\cdot 10^{5}$ N
-Other forces: $r1=p1\left[\frac{n\cdot 2l}{2\cdot 1}\frac{2l}{4}\right]$ $r1=6.2832\cdot 10^{5}$ N $r2=p2\left[\frac{n\cdot D2^{2}}{4}\right]$ $r2=0$
Momentum eqn.x: $r1\cdot cos\left[\frac{n}{4}\right] - rs + r2\cdot cos\left[\frac{n}{4}\right] - r \cdot o.v.v1\cdot cos\left[\frac{n}{4}\right]$ $r2=0$ $rx=6.4802\cdot 10^{5}$ N

Momentum eqn.y:
$$F1 \cdot \sin\left(\frac{\pi}{4}\right) + Fy - W - F2 \cdot \sin\left(\frac{\pi}{4}\right) = \rho \cdot Q \cdot V2 \cdot \sin\left(\frac{\pi}{4}\right) - \rho \cdot Q \cdot V1 \cdot \sin\left(\frac{\pi}{4}\right)$$

$$F_{Y} = \rho \cdot Q \cdot V_{2} \cdot \sin\left(\frac{\pi}{4}\right) - \rho \cdot Q \cdot V_{1} \cdot \sin\left(\frac{\pi}{4}\right) - \left(F_{1} \cdot \sin\left(\frac{\pi}{4}\right) - W - F_{2} \cdot \sin\left(\frac{\pi}{4}\right)\right) \qquad F_{Y} = -1.3713 \cdot 10^{5} N$$